Tro (ic Solutions

Traffic Solutions Pty Ltd

PROPOSED RESIDENTIAL SUBDIVISION, LOT 6 DP 244030 & LOT 9 DP 250425 DIAMOND BEACH ROAD, DIAMOND BEACH.

TRAFFIC IMPACT ASSESSMENT

March 2010

Ref: 09.10.050

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1. <u>INTRODUCTION</u>

This report has been prepared to accompany a Development Application to Greater Taree City Council for a proposed residential subdivision at Lot 6 DP 244030 & Lot 9 DP 250425 Diamond Beach Road, Diamond Beach. (Figure 1)

The subdivision proposes 89 residential lots with vehicle access for all of these lots via a new road which will link Edgewater Drive and Anniversary Drive, Diamond Beach.

This report examines the traffic implications of the proposed development and will assess the:

- Proposed access arrangements.
- Suitability of the internal road layout.
- Estimated traffic generation of the proposal.
- Impacts of the estimated traffic generation on the existing road network.



LOCATION

Fig 1

2. <u>PROPOSED DEVELOPMENT</u>

SITE

The subject site is known as Lot 6 DP 244030 and Lot 9 DP 250425 Diamond Beach Road and is located between Edgewater Drive and Anniversary Drive, Diamond Beach. Currently this land is mostly vacant with a single house fronting Diamond Beach Road.

DEVELOPMENT PROPOSAL

The subdivision proposes 89 residential lots with all lots gaining vehicle access via a new road system that will link Edgewater Drive and Anniversary Drive, Diamond Beach. The new link road between Edgewater and Anniversary Drives will create a minor collector road route for this area.

Edgewater Road joins Diamond Beach Road at a 'T' intersection which has been provided with an AUSTROADS type AUR auxiliary right turn treatment. Sight distance in each direction along Diamond Beach Road is very good. Photographs of the available sight lines are attached in appendix A.

Anniversary Drive links with Diamond Drive which provides access to Diamond Beach Road via a single lane 4 way roundabout at the junction with Panorama Drive.

A reduced copy of the proposed subdivision layout prepared by Lidbury, Summers & Whiteman, Consulting Surveyors, Planners and Engineers is reproduced on the following page.



3. <u>EXISTING CONDITIONS</u>

Diamond Beach Road serves a collector road function in this area, while Edgewater and Anniversary Drive serve as local roads by providing access for vehicles from the existing and proposed lots to Diamond Beach Road.

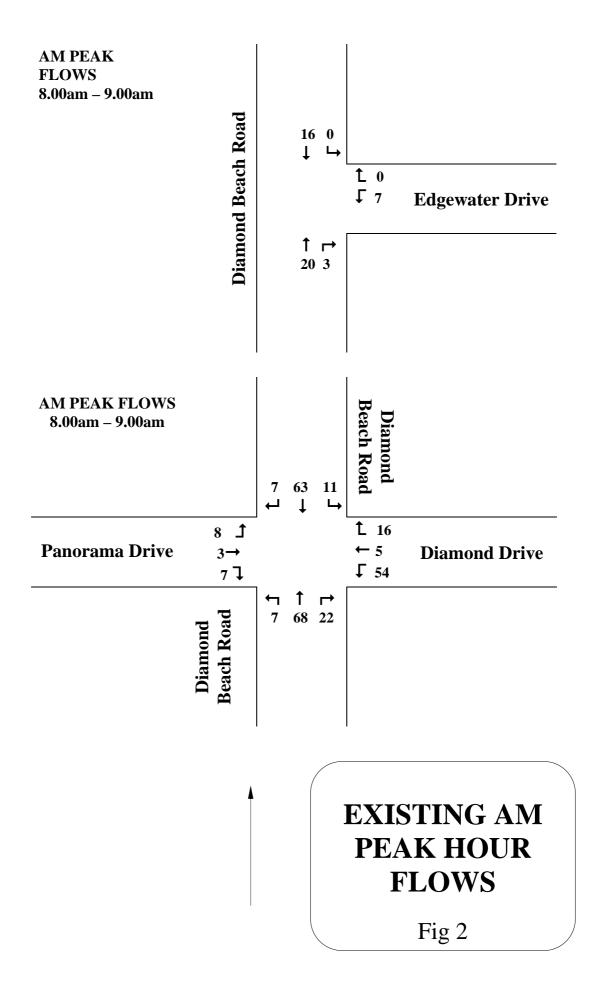
The main features of the existing traffic controls in the vicinity of the site are:

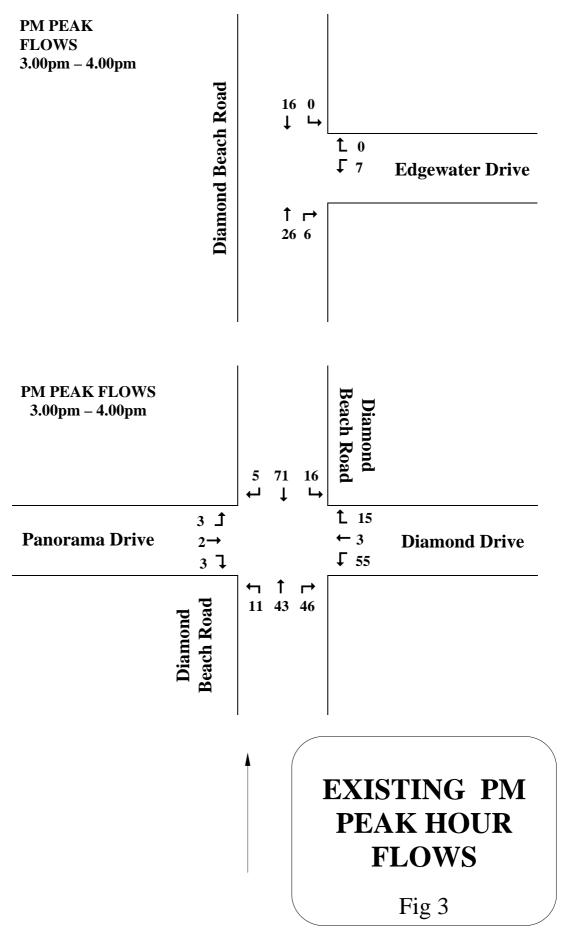
- A 50 Km/h speed limit on Diamond Beach Road.
- A 40 Km/h School Zone speed limit on Diamond Beach Road.
- A 60 Km/h speed limit on Diamond Beach Road at the intersection of Edgewater Drive.
- A 50 Km/h speed limit on Edgewater Drive.
- A 50 Km/h speed limit on Anniversary Drive.
- Give way restrictions in Edgewater Drive at Diamond Beach Road.
- Diamond Beach Road has a variable width pavement widening at intersections.
- Edgewater Drive has a variable pavement approximately 6.5m wide with no kerb.
- Anniversary Drive is 8m wide and provides kerb and gutter.

No restriction to on street car parking has been implemented in the vicinity of the subject site.

Data on the traffic movements in the vicinity of the subject site have been collected by surveys undertaken as part of this assessment by R.O.A.R Data on behalf of this firm from 7.00am - 9.00am and 3.00pm - 6.00pm on Wednesday 3rd March 2010 at the intersections of Diamond Beach Road and Edgewater Drive and the Roundabout at the intersection of Diamond Beach Road, Diamond Drive and Panorama Drive.

The detailed results of the surveys are attached as appendix B. The morning and evening peak hour flows at the intersection of Diamond Beach Road and Edgewater Drive were recorded as 8.00am -9.00am and 3.00pm -4.00pm. The morning and evening peak hour flows at the intersection of Diamond Beach Road, Diamond Drive and Panorama Drive were recorded at the same times, namely, as 8.00am -9.00am and 3.00pm -4.00pm. The peak hour flows at the intersections are depicted as Figure 2 and 3 on the following page.





4. <u>KEY ISSUES</u>

VEHICLE ACCESS

Vehicular access to the development will be available via Edgewater Drive or Anniversary Drive. The link road between these roads is proposed to have a carriageway width of 16m, i.e. an 8m pavement with 4.0m footpaths on both sides.

A site inspection reveals that intersections of Diamond Beach Road and Edgewater Drive and Diamond Drive, Anniversary Drive/Panorama Drive provide very good sight lines. The sight lines easily exceed the Safe Intersection Site Distance requirement of 90m for 50 km/h and 115m for 60km/h for rural areas suggested by the AUSTROADS Guide to Traffic Engineering Practice, Part 5 – intersections at grade. Attached as appendix A are photographs indicating the available sight lines.

INTERNAL ROAD LAYOUT

The internal roads proposed to serve the subdivision are generally 8m wide with the exception of the cul-de-sacs which are 6m wide and the east/west road to the south of the site which is 5.5m. All roads are sufficient to cater for two way vehicle flows.

To determine if a garbage truck can access and circulate through the site, the internal roads have been assessed using the AS 2890.2 – 2002, 8.8m long medium rigid vehicle turning template. This vehicle is typical of standard large garbage trucks used by garbage contractors and Council's alike. This assessment is carried out by overlaying the turning template upon the plans provided at scale revealing that there is sufficient roadway width to enable this vehicle to travel around the roadway and access all roads as required.

Consequently, the subject development proposal is sufficient to enable access and circulation by the Australian Standard medium rigid vehicle and provide for two way flows.

TRAFFIC

An estimation of the traffic generation of the proposed development can be calculated by reference to the Roads and Traffic Authority's 'Guide to Traffic Generating Developments, Section 3 - Landuse Traffic Generation' of October 2002. The guide specifies the following peak hour traffic generation rates for dwelling houses:

Daily vehicle trips = 9.0 per dwelling Weekday peak hour vehicle trips = 0.85 per dwelling Assuming only 1 dwelling per lot, the estimated traffic generation of this development calculates as:

89 lots/dwellings @ 0.85 trips/dwelling = 75.65 peak hour trips

Accordingly, the potential traffic generation of the proposed development is in the order of **76** vehicle trips in the peak hours.

This assessment will assume that 80% (61 trips) and 20% (15 trips) of the potential traffic generation will depart and approach the site in the morning peak hour respectively and that this situation will reverse in the evening peak hour.

Further, to assess the impact of the development on the intersections of Diamond Beach Road with Edgewater Drive and Diamond Drive the estimated morning and evening peak hour approach and departure vehicle trips have been assigned proportionally to these intersections on the basis of existing movements and logical vehicle routes. Figure 4 and 5 depicts the potential additional morning and afternoon peak hour flows at each intersection.

Using INTANAL a software programme developed by Sims Varley Pty Ltd for the purpose of analysing signalised, roundabout and sign controlled intersections, the effect of the estimated traffic generation of this development on the key intersections has been assessed.

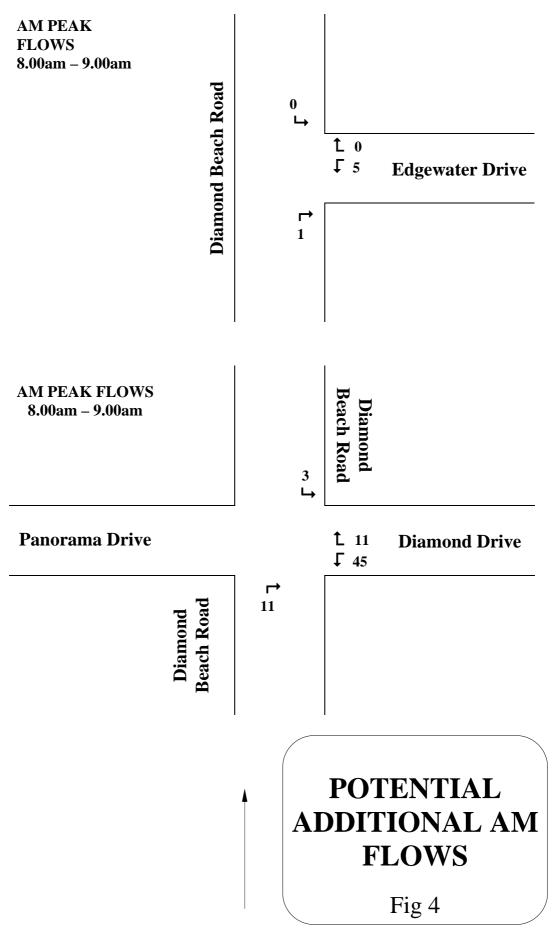
Tabled below are the results of the intersection modeling. Attached as appendix C are the INTANAL output files for the proposed scenarios and a brief guide on evaluating the results of INTANAL analysis is reproduced in the following pages:

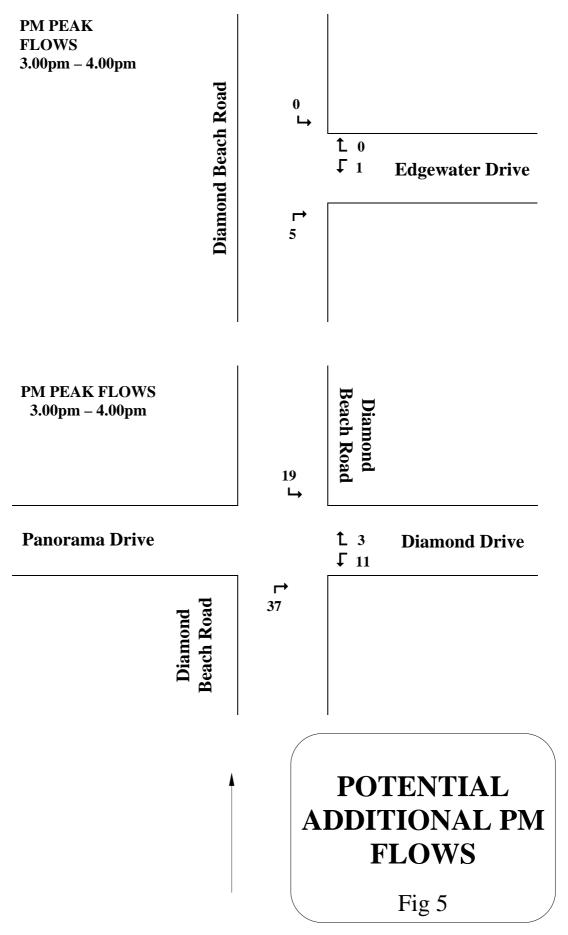
	Intersection of Ed	0	Diamond Beach Road Intersection)	l, Diamond Beach												
	Exi	sting	Prop	osed												
	AM	•														
Level of																
Service	A	A	A	A												
Degree of																
Saturation	0.01	0.01	0.01	0.01												
Total Average																
Delay (sec/veh)	4.7	4.8	4.9	4.8												

	Intersection of Di		Diamond Drive and (Roundabout)	Panorama Drive,												
	Exis	sting	Prop	osed												
	AM															
Level of																
Service	A	A	A	A												
Degree of																
Saturation	0.06	0.06	0.08	0.08												
Total Average																
Delay (sec/veh)	4.1	4.2	4.4	4.3												

The results of the INTANAL analysis reveals:

- The existing intersections modeled will continue to provide a very good level of service, with minimal delays and spare capacity with the estimated additional traffic generation of the proposed development.
- The additional traffic demand on the intersections as a consequence of the proposed development will only alter the Degree of Saturation and Total Average Delays minutely.





EVALUATION OF THE RESULTS OF INTANAL

LEVEL OF SERVICE

THE LEVEL OF SERVICE FOR TRAFFIC SIGNALS, ROUNDABOUTS AND SIGN CONTROL INTERSECTIONS IS SHOWN BELOW, THIS IS BASED ON THE AVERAGE DELAY IN SECONDS PER VEHICLE:

AVERAGE DELAY PER VEHICLE	LEVEL OF SERVICE	TRAFFIC SIGNALS & ROUNDABOUTS	SIGN CONTROL
< 14	A	GOOD	GOOD
15 - 28	В	GOOD WITH MINIMAL DELAYS AND SPARE CAPACITY	ACCEPTABLE DELAYS AND SPARE CAPACITY
29 - 42	С	SATISFACTORY WITH SPARE CAPACITY	SATISFACTORY BUT ACCIDENT STUDY REQUIRED
43 - 56	D	SATISFACTORY BUT OPERATING NEAR CAPACITY	NEAR CAPACITYAND ACCIDENT STUDY REQUIRED
57 - 70	Е	AT CAPACITY: AT SIGNALS INCIDENTS WILL CAUSE EXCESIVE DELAYS, ROUNDABOUTS REQUIRE ANOTHER CONTROL MODE	AT CAPACITY AND REQUIRES ANOTHER CONTROL MODE
> 70	F	UNSATISFACTORY	UNSATISFACTORY

DEGREE OF SATURATION

THE DEGREE OF SATURATION IS ANOTHER MEASURE OF THE OPERATIONAL PERFORMANCE OF INDIVIDUAL INTERSECTIONS.

For traffic signal controlled intersections both queue length and delay increase rapidly as the Degree of Saturation approaches 1.0, and it is usually attempted to keep it below 0.9.

For roundabouts or sign controlled intersections, oversaturation is indicated by a value in excess of 0.8.

AVERAGE VEHICLE DELAY

THE AVERAGE VEHICLE DELAY PROVIDES A MEASURE OF THE OPERATIONAL PERFORMANCE OF AN INTERSECTION AS INDICATED IN THE ABOVE TABLE . THE AVERAGE VEHICLE DELAYS IN THE TABLE SHOULD BE USED AS A GUIDE ONLY AS LONGER DELAYS COULD BE TOLERATED IN SOME LOCATIONS.

ENVIRONMENTAL CAPACITY

Utilising the traffic flow data recorded at the intersections of Diamond Beach Road with Edgewater Drive and Diamond Drive/Panorama Drive the traffic related environmental effect of the proposed residential subdivision on Edgewater Drive has been assessed. Edgewater Road and Diamond Drive currently serve a local road function in this area and the Roads and Traffic Authority provides a guide to the Environmental Capacity of residential streets in the 'Guide to Traffic Generating Developments, Section 4 – Interpretation of Traffic Impacts' of October 2002. This guide suggests a desirable environmental goal of 200 and maximum of 300 vehicles/hour for local roads.

As stated previously the current peak hour traffic volumes on Edgeworth Drive and Diamond Drive have been recorded by surveys undertaken as part of this assessment. The following table provides a comparison of the existing peak hour volumes, potential additional flows and the RTA Environmental Capacity value:

Location	Classification	Pea	ak Hou	r Volur	nes	RTA Suggested
		Exis	ting	Pote	ntial	Environmental Capacity
		AM	PM	AM	PM	
Edgewater Drive	Local Road	10	13	15	19	200 300 (max)
Diamond Drive	Local Road	111	137	181	207	200 300(max)

The survey results reveal that the existing traffic flows along Edgewater and Diamond Drives are well below the maximum RTA suggested Environmental Capacity and that the potential additional peak hour traffic flows of 76 vehicles estimated for the proposed development will not cause this value to be exceeded.

It should be noted that Environmental Capacity is not an indication of the number of vehicles that can travel along a roadway before congestion occurs but is the RTA's interpretation of when residents may raise concern over vehicle volumes.

6. <u>CONCLUSIONS</u>

The preceding analysis has revealed that:

- The available sight lines at the intersections of Diamond Beach Road with Edgewater Drive and Diamond Drive are considered to be satisfactory as they exceed the Safe Intersection Sight Distance requirements of AUSTROADS.
- All internal roads are sufficient in width to cater for two way traffic flows.
- The development proposal is sufficient to enable access and circulation by the Australian Standard medium rigid vehicle.
- The existing intersections of Diamond Beach Road with Edgewater Drive and Diamond/Panorama Drive will continue to provide a very good level of service, with minimal delays and spare capacity with the estimated additional traffic generation of the proposed development.
- The additional traffic demand on the intersection of Edgewater Drive and Diamond Beach Road, as a consequence of the proposed development will only alter the Degree of Saturation and Total Average Delays minutely.
- The potential 76 additional vehicle trips generated by this proposal will not cause the RTA suggested maximum Environmental Capacity volume for local roads to be exceeded in Edgewater Drive or Diamond Drive, Diamond Beach.

APPENDIX A – SITE PHOTOGRAPHS

Thoras Television Lagrange to District to

Photo 1 – View from Edgewater Drive to Diamond Beach Road





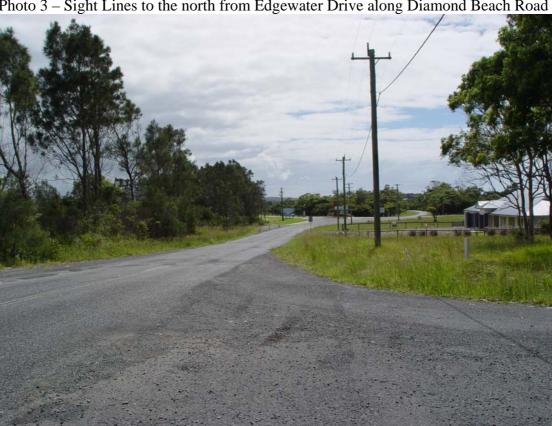
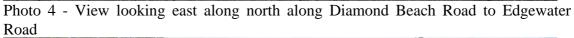


Photo 3 – Sight Lines to the north from Edgewater Drive along Diamond Beach Road





APPENDIX B – INTERSECTION COUNTS



R.O.A.R. DATA
Reliable, Original & Authentic Results
Ph.88196847, Fax 88196849.
Mobile.0418239019

Client : Traffic Solutions Job No/Name : 2996 Diamond Beach Counts Day/Date : Wednesday 3rd March 2010

				_	_				_
PEDS	NORTH	EAST	SOUTH		PEDS	NORTH	EAST	SOUTH	
Time Per	Diamond	Edgewater Rd	Diamond	TOT	Peak Per	Diamond	Edgewater Rd	Diamond	TOT
0700 - 0715	1	1	0	2	0700 - 0800	1	3	1	5
0715 - 0730	0	2	1	3	0715 - 0815	0	2	1	3
0730 - 0745	0	0	0	0	0730 - 0830	0	1	0	1
0745 - 0800	0	0	0	0	0745 - 0845	0	1	0	1
0800 - 0815	0	0	0	0	0800 - 0900	0	1	0	1
0815 - 0830	0	1	0	1					
0830 - 0845	0	0	0	0					
0845 - 0900	0	0	0	0					
Per End	1	4	1	6					

PEAK HR 0 1	0	1
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Lights	NO	RTH	E/	IST	SO	UTH	1	Heavies	NO	RTH	E/	IST	so	UTH	1	Combined	NO	RTH	E/	AST	SO	UTH	i
	Dian	nond	Edgew	ater Rd	Dian	nond	1		Dian	nond	Edgew	ater Rd	Dian	nond	1		Dian	nond	Edgew	ater Rd	Dian	nond	
Time Per	<u>T</u>	L	R	L	R	<u>T</u>	TOT	Time Per	<u>T</u>	L	<u>R</u>	L	<u>R</u>	<u>T</u>	TOT	Time Per	I	L	<u>R</u>	L	<u>R</u>	<u>I</u>	TOT
0700 - 0715	1	0	0	0	4	0	5	0700 - 0715	0	0	0	0	0	0	0	0700 - 0715	- 1	0	0	0	4	0	5
0715 - 0730	2	0	0	0	2	2	6	0715 - 0730	0	0	0	0	0	0	0	0715 - 0730	2	0	0	0	2	2	6
0730 - 0745	1	0	0	0	0	6	7	0730 - 0745	0	0	0	0	0	0	0	0730 - 0745	1	0	0	0	0	6	7
0745 - 0800	0	0	0	2	0	5	7	0745 - 0800	0	0	0	0	0	0	0	0745 - 0800	0	0	0	2	0	5	7
0800 - 0815	2	0	0	2	- 1	6	11	0800 - 0815	0	0	0	0	0	0	0	0800 - 0815	2	0	0	2	1	6	-11
0815 - 0830	4	0	0	2	- 1	2	9	0815 - 0830	0	0	0	0	0	0	0	0815 - 0830	4	0	0	2	- 1	2	9
0830 - 0845	2	0	0	3	0	8	13	0830 - 0845	0	0	0	0	0	0	0	0830 - 0845	2	0	0	3	0	8	13
0845 - 0900	8	0	0	0	1	4	13	0845 - 0900	0	0	0	0	0	0	0	0845 - 0900	8	0	0	0	1	4	13
Per End	20	0	0	9	9	33	71	Per End	0	0	0	0	0	0	0	Per End	20	0	0	9	9	33	71

Lights		RTH		ST		UTH]	Heavies		RTH	E/			UTH .]	Combined		RTH	EA			UTH	ĺ
	Dia	nond	Edgew	ater Hd	Dian	nond			Diai	nond	Edgew	ater Kd	Dian	nond			Diar	nond	Edgew	ater Rd	Dian	nond	
Peak Per	<u>T</u>	L	<u>R</u>	L	<u>R</u>	I	TOT	Peak Per	<u>I</u>	L	<u>R</u>	L	<u>R</u>	<u>I</u>	TOT	Peak Per	<u>I</u>	L	<u>R</u>	L	<u>R</u>	<u>T</u>	TOT
0700 - 0800	4	0	0	2	6	13	25	0700 - 0800	0	0	0	0	0	0	0	0700 - 0800	4	0	0	2	6	13	25
0715 - 0815	5	0	0	4	3	19	31	0715 - 0815	0	0	0	0	0	0	0	0715 - 0815	5	0	0	4	3	19	31
0730 - 0830	7	0	0	6	2	19	34	0730 - 0830	0	0	0	0	0	0	0	0730 - 0830	7	0	0	6	2	19	34
0745 - 0845	8	0	0	9	2	21	40	0745 - 0845	0	0	0	0	0	0	0	0745 - 0845	8	0	0	9	2	21	40
0800 - 0900	16	0	0	7	3	20	46	0800 - 0900	0	0	0	0	0	0	0	0800 - 0900	16	0	0	7	3	20	46

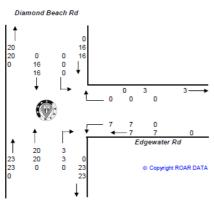
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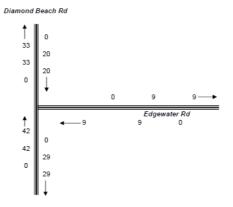


Client : Traffic Solutions Job No/Name : 2996 Diamond Beach Counts
Day/Date : Wednesday 3rd March 2010



TOTAL VOLUMES FOR COUNT PERIOD





Diamond Beach Rd

Diamond Beach Rd



R.O.A.R. DATA Reliable, Original & Authentic Results Ph.88196847, Fax 88196849. Mobile.0418239019

Client : Traffic Solutions Job No/Name : 2996 Diamond Beach Counts Day/Date : Wednesday 3rd March 2010

PEDS	NORTH	EAST	SOUTH	L	PEDS	NORTH	EAST	SOUTH	
Time Per	Panah Pd	Pd	Popoli Pd	TOT	Peak Per	Pozob Pd	Pd	Pozob Pd	TOT
1500 - 1515	0	0	0	0	1500 - 1600	0	1	0	1
1515 - 1530	0	0	0	0	1515 - 1615	0	1	0	1
1530 - 1545	0	0	0	0	1530 - 1630	0	1	0	1
1545 - 1600	0	1	0	1	1545 - 1645	0	1	0	1
1600 - 1615	0	0	0	0	1600 - 1700	0	0	0	0
1615 - 1630	0	0	0	0	1615 - 1715	0	0	0	0
1630 - 1645	0	0	0	0	1630 - 1730	0	0	0	0
1645 - 1700	0	0	0	0	1645 - 1745	0	0	0	0
1700 - 1715	0	0	0	0	1700 - 1800	0	0	0	0
1715 - 1730	0	0	0	0					
1730 - 1745	0	0	0	0	PEAK HR	0	1	0	1
1745 - 1800	0	0	0	0					
Per End	0	1	0	1					

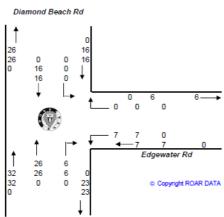
Lights	NO	RTH	E/	IST	SO	UTH	1	Heavies	NO	RTH	E	AST	SO	UTH	1	Combined	NO	RTH	E/	IST	SO	UTH	ı
	Diar	nond	Edge	water	Dian	nond			Dia	nond	Edge	water	Dian	nond	1		Diar	nond	Edge	water	Dian	nond	
Time Per	I	L	R	L	R	I	TOT	Time Per	Ι	L	R	L	R	I	TOT	Time Per	I	L	R	L	R	I	TOT
1500 - 1515	6	0	0	5	0	4	15	1500 - 1515	0	0	0	0	0	0	0	1500 - 1515	6	0	0	5	0	4	15
1515 - 1530	4	0	0	0	0	7	11	1515 - 1530	0	0	0	0	0	0	0	1515 - 1530	4	0	0	0	0	7	11
1530 - 1545	- 1	0	0	0	3	5	9	1530 - 1545	0	0	0	0	0	0	0	1530 - 1545	1	0	0	0	3	5	9
1545 - 1600	5	0	0	2	3	10	20	1545 - 1600	0	0	0	0	0	0	0	1545 - 1600	5	0	0	2	3	10	20
1600 - 1615	- 1	0	0	3	0	1	5	1600 - 1615	0	0	0	0	0	0	0	1600 - 1615	1	0	0	3	0	1	5
1615 - 1630	2	0	0	1	3	3	9	1615 - 1630	0	0	0	0	0	0	0	1615 - 1630	2	0	0	1	3	3	9
1630 - 1645	6	0	0	1	2	1	10	1630 - 1645	0	0	0	0	0	0	0	1630 - 1645	6	0	0	1	2	1	10
1645 - 1700	3	0	0	0	4	5	12	1645 - 1700	0	0	0	0	0	0	0	1645 - 1700	3	0	0	0	4	5	12
1700 - 1715	5	0	0	6	4	6	21	1700 - 1715	0	0	0	0	0	0	0	1700 - 1715	5	0	0	6	4	6	21
1715 - 1730	2	0	0	1	2	5	10	1715 - 1730	0	0	0	0	0	0	0	1715 - 1730	2	0	0	1	2	5	10
1730 - 1745	2	0	0	1	0	2	5	1730 - 1745	0	0	0	0	0	0	0	1730 - 1745	2	0	0	1	0	2	5
1745 - 1800	4	0	0	0	0	13	17	1745 - 1800	0	0	0	0	0	0	0	1745 - 1800	4	0	0	0	0	13	17
Per End	41	0	0	20	21	62	144	Per End	0	0	0	0	0	0	0	Per End	41	0	0	20	21	62	144
Lights	NO	RTH	F/	ST	SO	UTH	1	Heavies	NO	RTH	E/	AST	SO	UTH	1	Combined	NO	RTH	F/	ST	SO	UTH	1
		nond		water		nond	1			nond		water		nond	1			nond		water		nond	ı
Peak Per	I	Ŀ	<u>R</u>	L	<u>R</u>	<u> </u>	101	Peak Per	I	L	R	Ŀ	<u>R</u>	I	101	Peak Per	I	L	<u>R</u>	L	<u>R</u>	I	101
1500 - 1600	16	0	0	7	6	26	55	1500 - 1600	0	0	0	0	0	0	0	1500 - 1600	16	0	0	7	6	26	55
1515 - 1615	11	0	0	5	6	23	45	1515 - 1615	0	0	0	0	0	0	0	1515 - 1615	-11	0	0	5	6	23	45
1530 - 1630	9	0	0	6	9	19	43	1530 - 1630	0	0	0	0	0	0	0	1530 - 1630	9	0	0	6	9	19	43
1545 - 1645	14	0	0	7	8	15	44	1545 - 1645	0	0	0	0	0	0	0	1545 - 1645	14	0	0	7	8	15	44
1600 - 1700	12	0	0	5	9	10	36	1600 - 1700	0	0	0	0	0	0	0	1600 - 1700	12	0	0	5	9	10	36
101E 171E	4.0	0			42	4.5	62	101E 171E	0		0		^			404E 474E	4.0		0		12	4.5	52

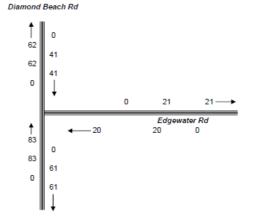


Client : Traffic Solutions Job No/Name : 2996 Diamond Beach Counts Day/Date : Wednesday 3rd March 2010

TOTAL VOLUMES FOR COUNT PERIOD

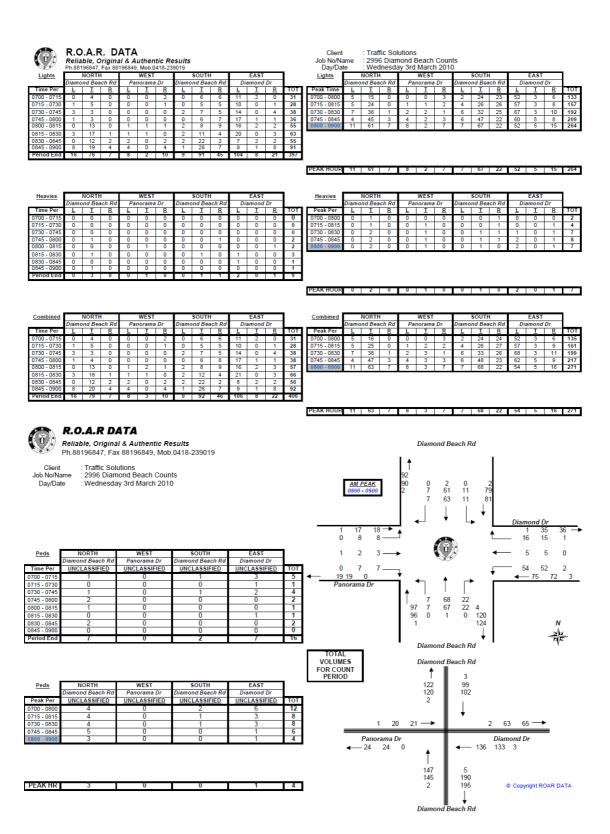


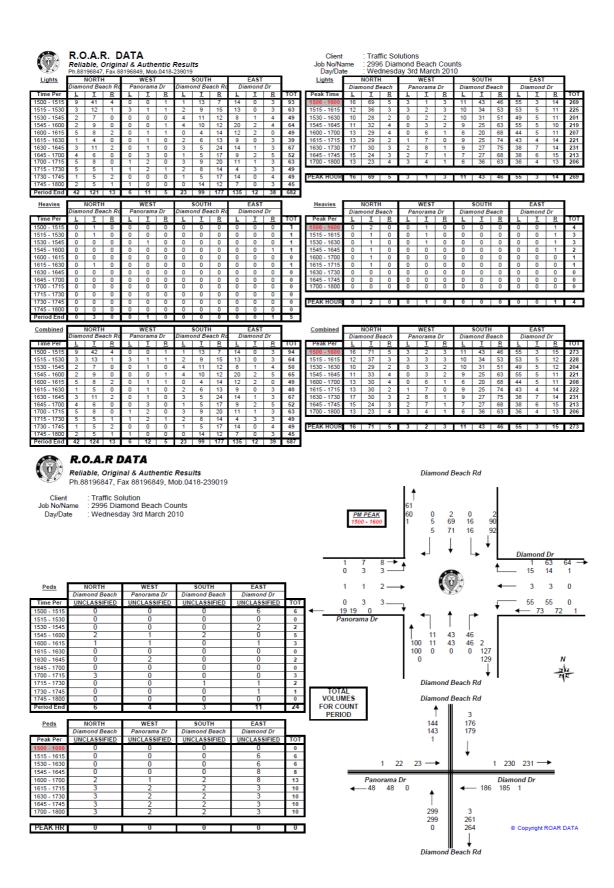




Diamond Beach Rd

Diamond Beach Rd





APPENDIX C – INTANAL RESULTS

VOLUME DATA SCREEN

AM	Vol		AM PE		Utrn	Vol		PM PEA		IItrn	۷ol		BUSINESS Phse Yv	al Utrn
1L													A 0. S 0. B 0.	
2T 2R 3L 3T 3R 4L 4T													В А А О.	
4R				A 1 2 3 4	5 5 5	ELT 4.0 4.0 4.0	H%AM 0 0 0	H%PM 0 0 0	H%B 0 0 0	L/S 0'	PD-L 0 0	PD-R 0 0	Sign Ho	N 25
				_	lle = 1									
PLATOO App 1 2 3 4	100		100	P% R(R(R(,	PEDE P#AN 0 0 0 0		U	U		WALK-Walk 0 0 0 0 0	-CLEAR C: 0 0 0 0	RANCE Lear	
								ra sci						
	Dov												Approa wn Lane	ch 4 s Grade
Type T2		0	2	0			1	0	0	1	()		
1		T 9	Lngth 9999 20	2000	Type L	Ln <u>g</u> 999	gth 8	Sat 5	Гуре Т	Lngtl	n Sat 2000	: Ту <u>г</u>)	pe Lngt	h Sat
8	Al h	No 1 M 0	Parkin PM 0	ng BUS 0	No AM 0 0	o Pai PN	rking 4 I 0	BUS 0	No AM 0	Park: PM 0	ing BUS	S AM	No Park M PM	ing BUS
Depar TCS# 0		Roui	ndaboı	ıt	0 R Ent 1	ounda	about		Roi	undabo	out		Roundak nt Cir	
File =														
				STOPS	- CYC	LE LE	ENGTH	- PH	ASE SI	PLITS	DATA	SCRE	EN	
Phse P A 6 B 3 C		CLo 50	AM PEA Yo 0.02	AK		PT%o 63.0 37.0	CLo 50	PM PEA Yo 0.02	AK		PT%o 60.0 40.0		BUSINESS Yo 0.02	3
D E F G	I	Delo 0	Delay	Sm= /m= /m=	0.02 0.02 0.27	A D	Delo 0	Delay	Sm= /m= /m=	0.02 0.02 0.35	7 10	Delo 0	DSm= Ym= Delaym=	140 0.02 0.02 0.02
S	ignal 0	ls .1	Signs	s Ro .0	ound :	Siana	als	Signs	s Ro	ound	Signa	als	Signs 0.0 0	Round 0.0 0

```
D/So 0.02 0.01
L/So A A
                         0.02
                                                                           0.01
                        A
                                                                          A
File = DIAEDGPR
                                 A RHT Lanes LHT Lanes
                                  Length No.Length No.
                                   10 1
                                                10 1
                                 3
                                 4
                             VOLUME DATA SCREEN
                                     PM PEAK
             AM PEAK
                                                               BUSINESS
      Vol Sat Phse Yval Utrn Vol Sat Phse Yval Utrn Vol Sat Phse Yval Utrn
                 A 0.04 43 1394
 1L
          163
                                   356
                                                            186
                                                       1667 1565
                                                                    A 1.07
 1T
       68 1587
                                          A 0.03
                                                     1667 1565 A 1.0.

51 47 S 1.09

146 296 B 0.49

1351 0.49
               S 0.04
S 0.07
B 0.04
0.00
                                        S 0.07
 1R
      59 826
                                                      146 296 B 0.49
1351 0.49
51 96 S 0.53
146 186 A 0.78
1667 1565 A 1 07
                              100 1651
                                        B 0.06
0.00
 2L
      65 1602
                                                   51
146 186
1667 1565
47
296
1351
                              1750
26 1515
 2Т
         1750
              S 0.02
A 0.05
A 0.05
                                        S 0.02
A 0.05
 2R
      19 978
      22 453
                               19 369
                                         A 0.05
                                                                   A 1.07
      63 1297
                               71 1381
                               148
583
1750
      149
 3R
                                         0.01
                 0.01
                                                                    0.49
0.49
 4L
         1000
        1750
 4T
 4R
          157
                   0.04
                                            0.06
                                   48
                                                             96
                                                                     0.53
                      A Min ELT H%AM H%PM H%B L/S PD-L PD-R Sign Hold LKph 1 5 4.0 0 0 0 0 0' 0 0 G N 25 3 5 4.0 0 0 0 0 0' 0 5 25
                                         0
                       File = DIADIAPR
                       Type = T2
                                                    WALK-CLEARANCE
PLATOON DATA
                              PEDESTRIAN VOLUME
App
      P%AM
               P%PM
                       Р%В
                               P#AM P#PM P#B
                                                       Walk Clear
                                       0 0
0 0
0 0
              R0
                              0
      R0
                       R0
 2
      R0
               R0
                       R0
 3
                       R0
 4
      R0
              R0
                      R0
                               0
                                       0
                                                       0
                                                              0
                             LANES DATA SCREEN
           Approach 1
                                                              Approach 4
                                              Approach 3
                            Approach 2
       Down Lanes Grade Down Lanes Grade Down Lanes Grade Down Lanes Grade
 Туре
               1
                      Ω
                                   1
                                        Ω
                                              0
                                                     1
 Т2
        Type Lngth Sat Type Lngth Sat Type Lngth Sat Type Lngth Sat LT 9999 1750 LT 9999 1750 LT 9999 1750
 Lane
   2
   3
   5
    6
          No Parking
                           No Parking
                                              No Parking
                                                               No Parking
        AM PM BUS
0 0 0
0 0 0
Roundabout
                         AM PM BUS
                                             AM PM BUS
                                                              AM PM BUS
                                0
                                                   0
                                       0
                                             0
                                                         0
 Apprch
                          0 0 0
Roundabout
                                             U 0 0
Roundabout
 Depart
                                                               Roundabout
         Ent Cir Wdth Ent Cir Wdth 1 1 7 1 1 7
 TCS#
                                             Ent Cir Wdth
                                                             Ent Cir Wdth
  0
File = DIADIAPR
```